

## MiTek USA, Inc.

7777 Greenback Lane Suite 109 Citrus Heights, CA, 95610 Telephone 916/676-1900 Fax 916/676-1909

Re: LC\_CL885D24 Plan 885D 4' Front Garage Ext 30#

The truss drawing(s) referenced below have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource - Colorado Springs.

Pages or sheets covered by this seal: R52082825 thru R52082836

My license renewal date for the state of Colorado is October 31, 2019.



October 23,2017

Hernandez, Marcos

**IMPORTANT NOTE:** Truss Engineer's responsibility is solely for design of individual trusses based upon design parameters shown on referenced truss drawings. Parameters have not been verified as appropriate for any use. Any location identification specified is for file reference only and has not been used in preparing design. Suitability of truss designs for any particular building is the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.



or bearing surface.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 369 lb uplift at joint 11 and 318 lb uplift at joint 1.

9) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

PE-46766 VIDNAL October 23,2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
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 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
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 **ANSUTP11 Quality Criteria, DSB-89 and BCSI Building Component** Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

# 7777 Greenback Lane Suite 109 Citrus Heights, CA 95610







	10-9-0					J	7-9-0		
1	18-9-0					1	9-0-0		1
Plate Offsets (X,Y)	[1:0-2-0,0-1-8], [6:0-3-0,0-3-0], [22:0-3	-0,0-3-0], [27:0-2-15,0-2-0], [	50:0-3-0,0-3-0	)]				1	
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL         30.0           TCDL         7.5           BCLL         0.0           BCDL         7.5	Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2009/TPI2007	TC 0.17 BC 0.14 WB 0.17 (Matrix)	Vert(LL) Vert(TL) Horz(TL)	-0.01 -0.01 0.01	28 28 29	n/r n/r n/a	120 120 n/a	MT20 Weight: 198 lb	197/144 FT = 0%
LUMBER- TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2			BRACING TOP CHO	- RD	Structu end ve	ural wood erticals.	I sheathing o	directly applied or 6-0-0	) oc purlins, except
OTHERS 2x4 SF	PF No.2		BOTCHO	ΝU	6-0-0 (	bc bracin	g: 55-56,44-	45,37-39,29-30.	

(lb) - Max Horz 1=269(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 44, 29, 43, 45, 46, 47, 48, 49, 50, 51, 52, 53, 54, 55, 42, 41, 40, 39, 37, 36, 35, 34, 33, 32, 31, 30 except 56=-111(LC 9)

Max Grav All reactions 250 lb or less at joint(s) 1, 44, 29, 43, 45, 46, 47, 48, 49, 50, 51, 52, 53, 54, 55, 42, 41, 40, 39, 37, 36, 35, 34, 33, 32, 31, 30 except 56=363(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 9-10=-106/255, 10-11=-97/293, 11-58=-88/325, 12-58=-84/330, 12-13=-80/373, TOP CHORD
  - 13-14=-68/397, 14-15=-60/388, 15-16=-53/347, 16-59=-39/305, 17-59=-43/300,

40.00

- 17-18=-35/267 2-56=-269/278
- WEBS

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-05; 100mph; TCDL=4.5psf; BCDL=4.5psf; h=15ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Corner(3) 0-0-0 to 3-0-0, Exterior(2) 3-0-0 to 19-9-0, Corner(3) 19-9-0 to 22-9-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Plates checked for a plus or minus 5 degree rotation about its center.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 44, 29, 43, 45, 46,
- 47, 48, 49, 50, 51, 52, 53, 54, 55, 42, 41, 40, 39, 37, 36, 35, 34, 33, 32, 31, 30 except (jt=lb) 56=111. 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 44, 43, 45, 46, 47, 48, 49, 50, 51, 52, 53, 54, 55, 56, 42, 41, 40, 39, 37, 36, 35, 34, 33, 32, 31, 30.
- 11) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss





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vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Plates checked for a plus or minus 5 degree rotation about its center.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Bearing at joint(s) 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=144, 18=572, 12=537.

7) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.



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- 3) All plates are MT20 plates unless otherwise indicated.
- 4) Plates checked for a plus or minus 5 degree rotation about its center.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Bearing at joint(s) 13, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 266 lb uplift at joint 13 and 317 lb uplift at joint 7.
  8) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

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<b>├</b> ──	12-0-0			32-0-0							
Plate Offsets (X,Y)	[18:0-3-0,0-3-0]			20-0-0							
LOADING (psf) TCLL 30.0 TCDL 7.5 BCLL 0.0 BCDL 7.5	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2009/TPI2007	<b>CSI.</b> TC 0.18 BC 0.09 WB 0.16 (Matrix)	DEFL. in Vert(LL) 0.01 Vert(TL) 0.01 Horz(TL) 0.01	(loc) l/defl L/d 24 n/r 120 24 n/r 120 23 n/a n/a	PLATES         GRIP           MT20         197/144           Weight: 170 lb         FT = 0%						
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF OTHERS 2x4 SF	PF No.2 PF No.2 PF No.2 PF No.2		BRACING- TOP CHORD BOT CHORD	BRACING-         TOP CHORD       Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.         BOT CHORD       Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 30-32,25-26.							
REACTIONS. All b. (lb) - Max H Max U Max G	earings 32-0-0. lorz 47=-280(LC 7) lplift All uplift 100 lb or less at joint(s) 4 29, 28, 27, 26, 25, 23 irav All reactions 250 lb or less at joint 32, 30, 29, 28, 27, 26, 23 except 25	7, 39, 40, 41, 42, 43, 44, (s) 47, 38, 39, 40, 41, 42 =367(LC 14)	45, 46, 37, 36, 35, 34, 3 , 43, 44, 45, 46, 37, 36, 3	33, 32, 30, 35, 34, 33,							
FORCES. (lb) - Max. TOP CHORD 6-7= 10-1 14-12 BOT CHORD 46-4' 41-4: 36-32 31-33: 26-2' WEBS 10-33'	Comp./Max. Ten All forces 250 (lb) -76/258, 7-49=-85/291, 8-49=-80/296, 4 1=-108/413, 11-12=-122/388, 12-50=-1 5=-148/270 7=-201/259, 45-46=-195/256, 44-45=-1 2=-197/257, 40-41=-197/257, 39-40=-1 7=-197/257, 35-36=-197/257, 34-35=-1 2=-194/257, 30-31=-197/256, 29-30=-1 7=-196/257, 25-26=-200/257, 23-25=-1 8=-257/72, 22-25=-279/228	or less except when show 3-9=-95/354, 9-10=-100/4 25/345, 13-50=-129/341 97/257, 43-44=-197/257 97/257, 38-39=-197/256 97/257, 28-29=-197/257 97/256	vn. 403, , 13-14=-139/308, , 42-43=-197/257, , 37-38=-197/256, , 32-33=-197/257, , 27-28=-197/257,								
NOTES- 1) Unbalanced roof liv 2) Wind: ASCE 7-05; ' heights) and C-C C exposed ; end vertie DCL=1.60 3) Truss designed for Gable End Details a 4) All plates are 2x4 M 5) Plates checked for 6) Gable requires conf 7) Gable studs spacec 8) This truss has been 9) Bearing at joint(s) 3 should verify capac 10) Provide mechanic 43, 44, 45, 46, 37, Continued on page 2	e loads have been considered for this of 100mph; TCDL=4.5psf; BCDL=4.5psf; orner(3) 0-1-12 to 3-1-12, Exterior(2) 3 cal left and right exposed;C-C for mem wind loads in the plane of the truss onl as applicable, or consult qualified buildi 1T20 unless otherwise indicated. a plus or minus 5 degree rotation about tinuous bottom chord bearing. d at 1-4-0 oc. a designed for a 10.0 psf bottom chord 18, 37, 36, 35, 34, 33, 32 considers partity of bearing surface. al connection (by others) of truss to bear, 36, 35, 34, 33, 32, 30, 29, 28, 27, 26, 30, 35, 34, 33, 32, 30, 29, 28, 27, 26, 30, 30, 30, 20, 20, 20, 20, 20, 20, 20, 20, 20, 2	lesign. 1=15ft; B=45ft; L=24ft; e: 2-1-12 to 12-0-0, Corner(3 pers and forces & MWFF y. For studs exposed to ng designer as per ANSI t its center. ive load nonconcurrent v allel to grain value using aring plate capable of wit 25, 23.	ave=2ft; Cat. II; Exp C; e 3) 12-0-0 to 15-0-0 zone; NS for reactions shown; L wind (normal to the face /TPI 1. vith any other live loads. ANSI/TPI 1 angle to grai thstanding 100 lb uplift at	nclosed; MWFRS (all cantilever left and right umber DOL=1.60 plate grip ), see Standard Industry in formula. Building designe t joint(s) 47, 39, 40, 41, 42,	PE-46766 October 23,2017						
WARNING - Verify of Design valid for use or a truss system. Before building design. Bracin is always required for a ways required for the system.	design parameters and READ NOTES ON THIS J hly with MiTek® connectors. This design is based use, the building designer must verify the applica ng indicated is to prevent buckling of individual tru- tability and to prevent buckling of individual tru-	AND INCLUDED MITEK REFER only upon parameters shown, a bility of design parameters and ss web and/or chord members onal injury and proparty domos	ENCE PAGE MII-7473 rev. 10/0 and is for an individual building upoperly incorporate this design only. Additional temporary and	03/2015 BEFORE USE. component, not n into the overall I permanent bracing direa the							

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Job	Truss	Truss Type	Qty	Ply	Plan 885D 4' Front Garage Ext 30#
LC_CL885D24	D2E	GABLE	1	1	R52082830
					Job Reference (optional)
Builders First Source,	Colorado Springs, CO, 80939			7.6	40 s Aug 16 2017 MiTek Industries, Inc. Mon Oct 23 07:47:37 2017 Page 2
			ID:sXZpu	SGpV3vG	qogiLi?QbrzchNW-4_h_ptR4kAoYsY8bkHfz7yxI5IGwx7S6d90ThhyQZlq

#### NOTES-

11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 47, 38, 39, 40, 41, 42, 43, 44, 45, 46, 37, 36, 35, 34, 33, 32, 30, 29, 28, 27, 26, 25. 12) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

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5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=213, 6=213.

6) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.



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	0-0 <u>-8</u> 0-0-8			<u>8-3-1</u> 8-2-9						
LOADIN TCLL TCDI	<b>G</b> (psf) 30.0 7 5	SPACING- 2-0-0 Plate Grip DOL 1.15	<b>CSI.</b> TC 0.24 BC 0.09	DEFL. Vert(LL)	in n/a n/a	(loc) -	l/defl n/a	L/d 999 999	PLATES MT20	<b>GRIP</b> 197/144
BCLL BCDL	0.0 7.5	Rep Stress Incr YES Code IRC2009/TPI2007	WB 0.03 (Matrix)	Horz(TL)	0.00	3	n/a	n/a	Weight: 20 lb	FT = 0%

BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

#### LUMBER-

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 OTHERS 2x4 SPF No.2

BOT CHORD2x4 SPF No.2OTHERS2x4 SPF No.2

REACTIONS. (lb/size) 1=163/8-2-1, 3=163/8-2-1, 4=303/8-2-1 Max Horz 1=-42(LC 7) Max Uplift 1=-44(LC 9), 3=-44(LC 9), 4=-31(LC 9)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=4.5psf; BCDL=4.5psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) 0-7-9 to 3-7-9, Interior(1) 3-7-9 to 4-1-8, Exterior(2) 4-1-8 to 7-1-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Plates checked for a plus or minus 5 degree rotation about its center.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 44 lb uplift at joint 1, 44 lb uplift at joint 3

and 31 lb uplift at joint 4.

7) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.



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2x4 ⋍

2x4 🛸

Structural wood sheathing directly applied or 4-3-1 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

0 <u>-0+8</u> 0-0-8			4-3-1 4-2-9					
Plate Offsets (X,Y)- [2:0-2-0,Edge]								
LOADING (psf) TCLL 30.0 TCDL 7.5 BCLL 0.0 BCDL 7.5	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IBC2009/TPI2007	CSI. TC 0.04 BC 0.08 WB 0.00 (Matrix)	<b>DEFL.</b> Vert(LL) Vert(TL) Horz(TL)	in (loc) n/a - n/a - 0.00 3	l/defl L/d n/a 999 n/a 999 n/a n/a	PLATES         GRIP           MT20         197/144           Weight: 9 lb         ET = 0%		

BRACING-

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD2x4 SPF No.2BOT CHORD2x4 SPF No.2

REACTIONS. (lb/size) 1=135/4-2-1, 3=135/4-2-1 Max Horz 1=18(LC 8) Max Uplift1=-25(LC 9), 3=-25(LC 9)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=4.5psf; BCDL=4.5psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Plates checked for a plus or minus 5 degree rotation about its center.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 25 lb uplift at joint 1 and 25 lb uplift at joint

´3.

7) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building designer. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 MSNITP11 Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



