

MiTek USA, Inc.

7777 Greenback Lane Suite 109 Citrus Heights, CA, 95610 Telephone 916/676-1900 Fax 916/676-1909

Re: 18-053858 Classic 220 B

The truss drawing(s) referenced below have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by BMC West-Fort Lupton, CO.

Pages or sheets covered by this seal: R54885974 thru R54885980

My license renewal date for the state of Colorado is October 31, 2019.



July 24,2018

Hernandez, Marcos

IMPORTANT NOTE: Truss Engineer's responsibility is solely for design of individual trusses based upon design parameters shown on referenced truss drawings. Parameters have not been verified as appropriate for any use. Any location identification specified is for file reference only and has not been used in preparing design. Suitability of truss designs for any particular building is the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.



vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33

TCLL: ASCE 7-10; Pf=30.0 psf (flat roof snow); Category II; Exp C; Fully Exp.; Ct=1.1
 Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 16.0 psf or 1.00 times flat roof load of 30.0 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=169, 7=249.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.







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MiTek 7777 Greenback Lane Suite 109 Citrus Heights, CA 95610



BOT CHORD

WEBS

Rigid ceiling directly applied or 7-11-0 oc bracing

3-6

1 Row at midpt

BOT CHORD 2x4 DF 1800F 1.6E or 2x4 DF No.1&Btr or 2x4 SPF 1650F 1.5E

2x4 DF 1800F 1.6E or 2x4 DF No.1&Btr or 2x4 SPF 1650F 1.5E *Except* WEBS 2-8,3-7: 2x4 SPF-S Stud

2x4 DF 1800F 1.6E or 2x4 DF No.1&Btr or 2x4 SPF 1650F 1.5E *Except* OTHERS 21-22: 2x4 SPF-S Stud WEDGE

Left: 2x4 SPF-S Stud

REACTIONS.	(lb/size)	6=1045/0-5-8, 1=969/0-4-0
	Max Horz	1=336(LC 11)
	Max Uplift	6=-242(LC 14), 1=-176(LC 10)
	Max Grav	6=1325(LC 21), 1=1015(LC 21

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 1-2=-2224/538, 2-3=-1283/319, 4-6=-477/211
- BOT CHORD 1-8=-709/2007, 7-8=-709/2007, 6-7=-408/1131
- WEBS 2-8=0/269, 2-7=-963/338, 3-7=-58/455, 3-6=-1361/380

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=4.5psf; BCDL=4.5psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) 0-3-8 to 3-3-8, Interior(1) 3-3-8 to 22-10-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry

Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) TCLL: ASCE 7-10; Pf=30.0 psf (flat roof snow); Category II; Exp C; Fully Exp.; Ct=1.1

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 16.0 psf or 1.00 times flat roof load of 30.0 psf on overhangs non-concurrent with other live loads

6) All plates are 2x4 MT20 unless otherwise indicated.

Gable studs spaced at 1-4-0 oc.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=242, 1=176.



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	7-10-8	15	-10-8			23-10-8		
Plate Offsets (X,Y) [1:	0-2-13,Edge], [1:0-0-0,0-1-6], [5:0-0-1:	2,0-1-11], [8:0-4-0,0-3-0]	-0-0			6-0-0		
LOADING (psf) TCLL 30.0 (Roof Snow=30.0) TCDL 7.5 BCLL 0.0 BCDL 7.5	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.56 BC 0.49 WB 0.37 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.15 7-8 -0.25 7-8 0.07 5	l/defi L/d >999 240 >999 180 n/a n/a	PLATES GRIP MT20 169/123 Weight: 85 lb FT = 12%		
LUMBER- TOP CHORD 2x4 DF 1800F 1.6E or 2x4 DF No.1&Btr or 2x4 SPF 1650F 1.5E BRACING- TOP CHORD 2x4 DF 1800F 1.6E or 2x4 DF No.1&Btr or 2x4 SPF 1650F 1.5E BOT CHORD 2x4 DF 1800F 1.6E or 2x4 DF No.1&Btr or 2x4 SPF 1650F 1.5E TOP CHORD BOT CHORD BOT CHORD Rigid ceiling directly applied or 3-8-13 oc purlins. WEBS 2x4 SPF-S Stud 2x4 SPF-S Stud								
REACTIONS. (Ib/size) 5=1138/0-5-8, 1=1055/0-4-0 Max Horz 1=-58(LC 19) Max Uplift 5=-233(LC 11), 1=-184(LC 10)								
FORCES. (lb) - Max. Co TOP CHORD 1-2=-23; BOT CHORD 1-8=-624 WEBS 3-7=-137	mp./Max. Ten All forces 250 (lb) or l 79/734, 2-3=-2073/654, 3-4=-2074/64(4/2176, 7-8=-369/1492, 5-7=-624/2177 1/669, 4-7=-500/236, 3-8=-132/682, 2-	ess except when shown.), 4-5=-2397/722) 8=-511/237						

NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=4.5psf; BCDL=4.5psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) 0-3-8 to 3-3-8, Interior(1) 3-3-8 to 12-0-0, Exterior(2) 12-0-0 to 15-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33

2) TCLL: ASCE 7-10; Pf=30.0 psf (flat roof snow); Category II; Exp C; Fully Exp.; Ct=1.1

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 16.0 psf or 1.00 times flat roof load of 30.0 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 5=233, 1=184.



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7777 Greenback Lane Suite 109 Citrus Heights, CA 95610

Job	Truss	Truss Type	Qty	Ply	Classic 220 B		
18-053858	B3RXE	Common Supported Gable	1	1	lah Deference (entional)	R54885978	
					Job Reference (optional)		
BMC WEST TRUSS AND C	PTON, CO 80621	8.130 s Mar 11 2018 MiTek Industries, Inc. Tue Jul 24 10:24:57 2018 Page 1					
			ID:sA0eYfSdF	EL5plIF1A	InQ4vBLJi-KgaDS3OFIRn3cC21K2K3 5Til8m1Xps?vCcr	otMvutaa	
1	11-10-8	1			23-10-8	24-8-8	
11-10-8			12-0-0 0-1				

Scale = 1:40.0



				23-10-8					
Diata Offacta (X	(V) [1·	0 2 12 Edgel [1:0 0 0 0 1 2] [20:0 2 0	0 2 01	23-10-8					
	<u>, τ) [1.</u>	0-2-13,⊏ugej, [1.0-0-0,0-1-2], [29.0-3-0	7,0-3-0]						
LOADING (psf TCLL 3 (Roof Snow=30 TCDL BCLL BCDL	f) 30.0).0) 7.5 0.0 7.5	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.07 BC 0.03 WB 0.03 Matrix-S	DEFL. Vert(LL) -0.0 Vert(CT) 0.0 Horz(CT) 0.0	in (loc) 10 17 0 17 0 17	l/defl n/r n/r n/a	L/d 120 90 n/a	PLATES MT20 Weight: 115 lb	GRIP 169/123 FT = 12%
LUMBER- TOP CHORD BOT CHORD OTHERS	2x4 DF 18 2x4 DF 18 2x4 DF 18	800F 1.6E or 2x4 DF No.1&Btr or 2x4 S 800F 1.6E or 2x4 DF No.1&Btr or 2x4 S 800F 1.6E or 2x4 DF No.1&Btr or 2x4 S	SPF 1650F 1.5E SPF 1650F 1.5E SPF 1650F 1.5E	BRACING- TOP CHORD BOT CHORD	Structur Rigid ce	al wood s iling diree	sheathing dire ctly applied or	ctly applied or 6-0-0 o 10-0-0 oc bracing.	oc purlins.

WEDGE Left: 2x4 SPF-S Stud

REACTIONS. All bearings 23-10-8.

(lb) - Max Horz 1=-58(LC 15) Max Uplift All uplift 100 lb or less at joint(s) 27, 28, 29, 30, 31, 32, 33, 25, 24, 23, 22, 21, 20, 19, 17, 1

Max Grav All reactions 250 lb or less at joint(s) 26, 27, 28, 29, 30, 31, 32, 33, 25, 24, 23, 22, 21, 20, 19, 17, 1

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=4.5psf; BCDL=4.5psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Corner(3) 0-1-8 to 3-1-8, Exterior(2) 3-1-8 to 12-0-0, Corner(3) 12-0-0 to 15-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 3) TCLL: ASCE 7-10; Pf=30.0 psf (flat roof snow); Category II; Exp C; Fully Exp.; Ct=1.1
- 4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 16.0 psf or 1.00 times flat roof load of 30.0 psf on overhangs non-concurrent with other live loads.

- 6) All plates are 2x4 MT20 unless otherwise indicated. 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 1-4-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 27, 28, 29, 30, 31, 32, 33, 25, 24, 23, 22, 21, 20, 19, 17, 1.
- ΌΝΑΙ July 24,2018

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BMC WEST TRUSS AND COMPONENTS,



3x6 =

Plate Offsets (X,Y) [15	:0-3-8,0-3-0], [25:0-2-14,0-1-8]							
LOADING (psf) TCLL 30.0 (Roof Snow=30.0) TCDL 7.5 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IBC2015/TPI2014	CSI. TC 0.12 BC 0.08 WB 0.45 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) 0.00 13 0.00 13 -0.01 15	l/defl n/r n/r n/a	L/d 120 90 n/a	PLATES MT20 Weight: 101 lb	GRIP 169/123 FT = 12%
BCDL 7.5		Widuitx-O					Weight. 101 lb	1 1 - 12 /0
LUMBER- TOP CHORD 2x4 DF 18 BOT CHORD 2x4 DF 18 WEBS 2x4 DF 18 13-15: 2x6 12-15: 2x4 OTHERS 2x4 DF 18	00F 1.6E or 2x4 DF No.1&Btr or 2x4 3 00F 1.6E or 2x4 DF No.1&Btr or 2x4 3 00F 1.6E or 2x4 DF No.1&Btr or 2x4 3 0DF 1.6E or 2x4 DF No.1&Btr or 2x6 3 0DF 1.6E or 2x4 DF No.1&Btr or 2x4 3 00F 1.6E or 2x4 DF No.1&Btr or 2x4 3	SPF 1650F 1.5E SPF 1650F 1.5E SPF 1650F 1.5E *Except* SPF 1650F 1.5E SPF 1650F 1.5E	BRACING- TOP CHORE BOT CHORE) Structur except e) Rigid ce	al wood sl end vertica illing direc	neathing din Ils. Ily applied c	ectly applied or 6-0-0 o	oc purlins,
REACTIONS. All bearin (Ib) - Max Horz Max Uplift Max Grav	ngs 14-5-0. 25=275(LC 11) All uplift 100 lb or less at joint(s) 25. except 15=-512(LC 11) All reactions 250 lb or less at joint(s 17, 15 except 16=422(LC 11)	20, 21, 22, 23, 24, 19, 18) 25, 20, 21, 22, 23, 24, 19	, 17 9, 18,					
FORCES. (lb) - Max. Co TOP CHORD 2-25=-19 BOT CHORD 24-25=-4 19-20=-4 WEBS 3-25=-30	mp./Max. Ten All forces 250 (lb) or l 94/328, 10-11=-115/259, 11-12=-112/2 194/303, 23-24=-486/297, 22-23=-490 190/300, 18-19=-489/299, 17-18=-490 14/163, 12-16=-827/404, 12-15=-486/9	ess except when shown. 64 /300, 21-22=-489/299, 20- /300, 16-17=-487/297, 15- /62	21=-490/300, 16=-497/305					
 NOTES- 1) Wind: ASCE 7-10; Vult- MWFRS (envelope) and vertical left and right ex 2) Truss designed for wind Gable End Details as and 3) TCLL: ASCE 7-10; Pf=2 4) Unbalanced snow loads 5) This truss has been des non-concurrent with oth 6) All plates are 2x4 MT2Q 7) Gable requires continud 8) Truss to be fully sheath 9) Gable studs spaced at 1 10) This truss has been defined 11) Bearing at joint(s) 25, formula. Building des 12) Provide mechanical cr 23, 24, 19, 18, 17 exc 13) Beveled plate or shim 	=130mph (3-second gust) Vasd=103m d C-C Corner(3) -0-10-0 to 1-10-8, Ext posed;C-C for members and forces & l loads in the plane of the truss only. I policable, or consult qualified building 80.0 psf (flat roof snow); Category II; E is have been considered for this design signed for greater of min roof live load er live loads. unless otherwise indicated. bus bottom chord bearing. ed from one face or securely braced a 1-4-0 oc. esigned for a 10.0 psf bottom chord liv 20, 21, 22, 23, 24, 19, 18, 17, 16, 15 of igner should verify capacity of bearing onnection (by others) of truss to bearin ept (ijt=Ib) 15=512. required to provide full bearing surface	ph; TCDL=4.5psf; BCDL=- erior(2) 1-10-8 to 15-3-0 zv MWFRS for reactions show For studs exposed to wind designer as per ANSI/TPI xp C; Fully Exp.; Ct=1.1 of 16.0 psf or 1.00 times fl gainst lateral movement (i re load nonconcurrent with considers parallel to grain surface. In glate capable of withsta e with truss chord at joint(s	4.5psf; h=25ft; Cz one; cantilever le wn; Lumber DOL (normal to the fa 1. at roof load of 30 .e. diagonal web) any other live loz value using ANSI nding 100 lb uplif s) 20, 21, 22, 23.	at. II; Exp C; er ft and right exp =1.33 plate gri ce), see Stand .0 psf on overf ads. /TPI 1 angle to t at joint(s) 25, 24, 19, 18, 17	polosed; en p DOL=1. ard Indust hangs o grain 20, 21, 22 , 16, 15.	d 33 ry 2,	. MARC.	RADO LICENS S. HERN PE-46766 SIONAL SIONAL V 24.2018

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Scale = 1:42.7



			I			4-4-8					I	
LOADING (ps TCLL (Roof Snow=3 TCDL	sf) 30.0 ;0.0) 7.5	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.27 0.12 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.02 -0.03 0.00	(loc) 2-5 2-5 5	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 169/123
BCDL	7.5	Code IRC2015/TP	912014	Matri	x-P						Weight: 15 lb	FT = 12%
LUMBER- TOP CHORD	2x4 DF 180	00F 1.6E or 2x4 DF No.1	&Btr or 2x4 S	SPF 1650F 1	5E	BRACING- TOP CHORI	D 5	Structura	al wood s	heathing dir	ectly applied or 4-4-8	oc purlins,

BOT CHORD 2x4 DF 1800F 1.6E or 2x4 DF No.1&Btr or 2x4 SPF 1650F 1.5E WEBS 2x4 SPF-S Stud

except end verticals. BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 5=196/Mechanical, 2=265/0-3-8 Max Horz 2=72(LC 11) Max Uplift 5=-78(LC 14), 2=-112(LC 10) Max Grav 5=218(LC 21), 2=272(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=4.5psf; BCDL=4.5psf; h=25ft; Cat. II; Exp C; partially;
- MWFRS (envelope) and C-C Exterior(2) -0-10-0 to 2-2-0, Interior(1) 2-2-0 to 4-6-0 zone; cantilever left and right exposed ; end
- vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
- 2) TCLL: ASCE 7-10; Pf=30.0 psf (flat roof snow); Category II; Exp C; Fully Exp.; Ct=1.1
- 3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 16.0 psf or 1.00 times flat roof load of 30.0 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5 except (jt=lb) 2=112.



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